



The Farach Group

449 Web Foot Lane
Stevensville, MD 21666

Design Considerations:

1. Design uses ACI Code, Latest Edition...
2. Floor Slab Simulated on STAAD.Pro Release 2006...or RISA 3D-7.0
3. Floor Area Modeled is main project area...
4. Weight of Concrete assumed at 144 pcf...
5. Steel and Concrete Strengths are 40,000 and 3,000 psi respectively...UON
6. Work with Model's Sketch for Member and Joints Identification...

Loads from Modeling Analysis or Calculations:

Design Loads and General Arrangements			
Member No. =====▶ M1076			
Col Line→	Z1-7	Col Line→	Z1-8
DEAD LOADS			
Total DL Negative Moment =	54.83 k-ft	Total DL Negative Moment =	49.58 k-ft
Total DL Shear =	22.01 kips	Total DL Shear =	22.37 kips
Total DL Positive Moment =	63.54 k-ft		
LIVE LOADS			
Total LL Negative Moment =	22.00 k-ft	Total LL Negative Moment =	22.35 k-ft
Total LL Shear =	9.65 kips	Total LL Shear =	9.70 kips
Total LL Positive Moment =	25.14 k-ft		
General Slab Information			
Reinforcement Yield =	40.00 ksi	Slab Thickness =	9.75 inches
Concrete Strength =	3.00 ksi	Design d =	8.75 inches

General Distribution of Moment and Reinforcement:

Col Line→	Z1-7	Bay Width→	20.00 feet	Col Line→	Z1-8																																										
<table border="1"> <thead> <tr> <th colspan="2">Reinforcement</th> <th colspan="2">Reinforcement</th> <th colspan="2">Reinforcement</th> </tr> </thead> <tbody> <tr> <td>As(strip)=</td> <td>6.82</td> <td>As(strip)=</td> <td>3.41</td> <td>As(strip)=</td> <td>6.82</td> </tr> <tr> <td>ρ=</td> <td>0.0065</td> <td>ρ=</td> <td>0.0032</td> <td>ρ=</td> <td>0.0065</td> </tr> <tr> <td>As(mid)=</td> <td>2.48</td> <td>As(mid)=</td> <td>2.48</td> <td>As(mid)=</td> <td>2.48</td> </tr> <tr> <td>ρ=</td> <td>0.0024</td> <td>ρ=</td> <td>0.0024</td> <td>ρ=</td> <td>0.0024</td> </tr> <tr> <td></td> <td></td> <td>ρ(min)=</td> <td>0.0050</td> <td></td> <td></td> </tr> <tr> <td></td> <td></td> <td>ρ(max)=</td> <td>0.0232</td> <td></td> <td></td> </tr> </tbody> </table>						Reinforcement		Reinforcement		Reinforcement		As(strip)=	6.82	As(strip)=	3.41	As(strip)=	6.82	ρ =	0.0065	ρ =	0.0032	ρ =	0.0065	As(mid)=	2.48	As(mid)=	2.48	As(mid)=	2.48	ρ =	0.0024	ρ =	0.0024	ρ =	0.0024			ρ (min)=	0.0050					ρ (max)=	0.0232		
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Member	Col Line	Mid Span	Col Line	Col Line	Col Line	Member	Col Line	Mid Span	Col Line
	TL Neg Mom	TL Pos Mom	TL Neg Mom	Shear	Shear		Col Mom	Strp Mom	Col Mom
	Col Mom	Strp Mom	Col Mom				Col Mom	Strp Mom	Col Mom
	Mid Mom	Mid Mom	Mid Mom				Mid Mom	Mid Mom	Mid Mom
M1076	Z1-7		Z1-8	Z1-7	Z1-8		Mu → CAPACITIES		
Total Load	76.8	88.7	71.9	31.7	32.1	Ultimate	Z1-7		Z1-8
DL	41.1	38.1	37.2	22.0	22.4	LLu	26.4	24.1	26.8
DL	13.7	25.4	12.4			LLu	8.8	16.1	8.9
LL	16.5	15.1	16.8	9.7	9.7	TLu	75.7	69.9	71.4
LL	5.5	10.1	5.6			TLu	17.6	46.6	23.8
TL Col	57.6	53.2	53.9	Capacity →		TLu Col	169.9	87.2	169.9
TL Mid	19.2	35.5	18.0	Capacity →		TLu Mid	63.9	63.9	63.9



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Member No. =====▶				M1076		MuDL - Totals		Mu BALANCED CAPACITIES	
Col Line	Z1-7	Col Mom	16.5	X	1.2=	49.3	k-ft	ΔCol Mom=	94.1 k-ft
		Mid Mom	5.5	X	1.2=	8.8	k-ft	ΔMid Mom=	46.3 k-ft
Mid Span		Strp Mom	15.1	X	1.2=	45.7	k-ft	ΔStrp Mom=	17.3 k-ft
		Mid Mom	10.1	X	1.2=	30.5	k-ft	ΔMid Mom=	17.3 k-ft
Col Line	Z1-8	Col Mom	16.8	X	1.2=	44.6	k-ft	ΔCol Mom=	98.4 k-ft
		Mid Mom	5.6	X	1.2=	14.9	k-ft	ΔMid Mom=	40.1 k-ft

General Reinforcement Distribution...Worksheet.

Col Line →	Z1-7	Col Strip	Z1-8	← Col Line
Reinforcement 1		Reinforcement 3		Reinforcement 5
As(strip)= 1.86		As(strip)= 1.86		As(strip)= 1.86
		Reinforcement 6		
		As(strip)= 1.55		
Reinforcement 2				Reinforcement 4
As(strip)= 3.10				As(strip)= 3.10
Total Steel		Total Steel		Total Steel
As(strip)= 6.82		As(strip)= 3.41		As(strip)= 6.82

For Shear Design Check...

MDL= 56.30 k-ft		MDL= 45.23 k-ft		
VDL= 28.37 kips	← From adjacent Span	VDL= 18.96 kips		
MLL= 22.27 k-ft	From adjacent Span →	MLL= 22.47 k-ft		
VLL= 9.71 kips		VLL= 9.80 kips		
Col Line →	Z1-7	Mid Strip	Z1-8	← Col Line
Reinforcement 7		Reinforcement 9		Reinforcement 11
As(mid)= 1.24		As(mid)= 1.24		As(mid)= 1.24
		Reinforcement 12		
		As(mid)= 1.24		
Reinforcement 8				Reinforcement 10
As(mid)= 0.00				As(mid)= 0.00
Total Steel		Total Steel		Total Steel
As(mid)= 2.48		As(mid)= 2.48		As(mid)= 2.48
MDL= 50.23 k-ft	← From adjacent Bay →	MDL= 43.90 k-ft		
VDL= 28.37 kips		VDL= 18.96 kips		
MLL= 21.97 k-ft	← From adjacent Bay →	MLL= 21.97 k-ft		
VLL= 9.67 kips		VLL= 9.67 kips		
MDL= 47.18 k-ft	← From other adjacent Bay →	MDL= 47.12 k-ft		
VDL= 25.15 kips		VDL= 21.01 kips		
MLL= 22.03 k-ft	← From other adjacent Bay →	MLL= 22.00 k-ft		
VLL= 9.67 kips		VLL= 9.67 kips		



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Shear Design Check...Worksheet.

M1u= 2.20 k-ft	Moments & Shear for Punching Shear Calculations	M1u= 5.03 k-ft
M2u= 3.56 k-ft		M2u= 4.03 k-ft
Vu= 186.60 kips		Vu= 159.70 kips
Len of Col= 48.00 ins		Len of Col= 48.00 ins
Column Width → Z1-7	Span of Member → M1076	Z1-8 ← Column Width
Width of Col= 48.00 ins		Width of Col= 48.00 ins
Col Type= Interior...		Col Type= Interior...
Vc= 385.3 kips		Vc= 385.3 kips
Ac= 1,986.3 in ²		Ac= 1,986.3 in ²
Jc1= 1,072,477 in ³		Jc1= 1,072,477 in ³
Jc2= 1,072,477 in ³		Jc2= 1,072,477 in ³
vc= 194.00 psi		vc= 194.00 psi
vu= 95.77 psi		vu= 83.28 psi

Summary...

Middle Strip		
Mu(Cap)= 31.95 k-ft	Mu(Cap)= 31.95 k-ft	Mu(Cap)= 31.95 k-ft
0.125Mu= 12.62 k-ft	0.20Mu= 23.29 k-ft	0.125Mu= 11.91 k-ft
Moments → k-ft		
vc= 194.00 psi		vc= 194.00 psi
Mu(Cap)= 169.88 k-ft	Column Strip	Mu(Cap)= 169.88 k-ft
0.75Mu= 75.75 k-ft	MEMBER M1076	0.75Mu= 71.44 k-ft
vu= 95.77 psi		vu= 83.28 psi
Moments → k-ft		
Mu(Cap)= 31.95 k-ft	Mu(Cap)= 31.95 k-ft	Mu(Cap)= 31.95 k-ft
0.125Mu= 12.62 k-ft	0.20Mu= 23.29 k-ft	0.125Mu= 11.91 k-ft
Middle Strip		